PROJECT: DF18312.2023011.PR REFERENCE: DF18312.2023011.PR

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	DF18312.2023011.PR	1	

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

COUNTY CLEVELAND

PROJECT DESCRIPTION REPLACE BRIDGE NO. 0262 ON SR 1529 (MT ZION CHURCH RD) OVER FIRST BROAD RIVER

SITE DESCRIPTION BRIDGE NO. 0262

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CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

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NOTES:
1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
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DATE DECEMBER 2025

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BAB66070E9D747C...
SIGNATURE DATE

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT REFERENCE NO.	SHEET NO.
DF18312.2023011.PR	2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION

SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,
VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6

SOIL LEGEND AND AASHTO CLASSIFICATION

GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)							SILT-CLAY MATERIALS (> 35% PASSING #200)				ORGANIC MATERIALS		
GROUP CLASS.	A-1		A-3	A-2				A-4	A-5	A-6	A-7	A-1, A-2 A-3	A-4, A-5 A-6, A-7	
	A-1-a	A-1-b		A-2-4	A-2-5	A-2-6	A-2-7				A-7-5 A-7-6			
SYMBOL														
% PASSING #10	50 MX											GRANULAR SOILS	SILT-CLAY SOILS	MUCK, PEAT
#40	30 MX	50 MX	51 MN											
#200	15 MX	25 MX	10 MX	35 MX	35 MX	35 MX	35 MX	36 MN	36 MN	36 MN	36 MN			
MATERIAL PASSING #40 LL	—		—	40 MX	41 MN	40 MX	41 MN	40 MX	41 MN	40 MX	41 MN	SOILS WITH LITTLE OR MODERATE AMOUNTS OF ORGANIC MATTER		HIGHLY ORGANIC SOILS
PI	6 MX		NP	10 MX	10 MX	11 MN	11 MN	10 MX	10 MX	11 MN	11 MN			
GROUP INDEX	0		0	0		4 MX		8 MX	12 MX	16 MX	NO MX			
USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS. GRAVEL, AND SAND		FINE SAND	SILTY OR CLAYEY GRAVEL AND SAND				SILTY SOILS		CLAYEY SOILS				
GEN. RATING AS SUBGRADE	EXCELLENT TO GOOD							FAIR TO POOR				FAIR TO POOR	POOR	UNSUITABLE

PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30

CONSISTENCY OR DENSENESS

PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTENCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH ($TONS/FT^2$)
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	< 4 4 TO 10 10 TO 30 30 TO 50 > 50	N/A
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	< 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 > 30	< 0.25 0.25 TO 0.5 0.5 TO 1.0 1 TO 2 2 TO 4 > 4

TEXTURE OR GRAIN SIZE

U.S. STD. SIEVE SIZE	4	10	40	60	200	270
OPENING (MM)	4.76	2.00	0.42	0.25	0.075	0.053

BOULDER (BLDR.)	COBBLE (COB.)	GRAVEL (GR.)	COARSE SAND (CSE. SD.)	FINE SAND (F SD.)	SILT (SL.)	CLAY (CL.)

GRAIN SIZE MM: 305 | 75 | 2.0 | 0.25 | 0.05 | 0.005
GRAIN SIZE IN.: 12 | 3

SOIL MOISTURE - CORRELATION OF TERMS

SOIL MOISTURE SCALE (ATTERBERG LIMITS)	FIELD MOISTURE DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION
	- SATURATED - (SAT.)	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE
LL — LIQUID LIMIT PLASTIC RANGE (PI) PL — PLASTIC LIMIT	- WET - (W)	SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE
OM — OPTIMUM MOISTURE SL — SHRINKAGE LIMIT	- MOIST - (M)	SOLID; AT OR NEAR OPTIMUM MOISTURE
	- DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE

PLASTICITY

	PLASTICITY INDEX (PI)	DRY STRENGTH
NON PLASTIC	0-5	VERY LOW
SLIGHTLY PLASTIC	6-15	SLIGHT
MODERATELY PLASTIC	16-25	MEDIUM
HIGHLY PLASTIC	26 OR MORE	HIGH

COLOR

DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.

GRADATION

WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.
UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.
GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.

ANGULARITY OF GRAINS

THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.

MINERALOGICAL COMPOSITION

MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.

COMPRESSIBILITY

SLIGHTLY COMPRESSIBLE	LL < 31
MODERATELY COMPRESSIBLE	LL = 31 - 50
HIGHLY COMPRESSIBLE	LL > 50

PERCENTAGE OF MATERIAL

ORGANIC MATERIAL	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL	
TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE	1 - 10%
LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE	10 - 20%
MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME	20 - 35%
HIGHLY ORGANIC	> 10%	> 20%	HIGHLY	35% AND ABOVE

GROUND WATER

- WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING
- STATIC WATER LEVEL AFTER 24 HOURS
- PW PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA
- SPRING OR SEEP

MISCELLANEOUS SYMBOLS

- ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION
- SOIL SYMBOL
- ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT
- INFERRED SOIL BOUNDARY
- INFERRED ROCK LINE
- ALLUVIAL SOIL BOUNDARY
- 25/025 DIP & DIP DIRECTION OF ROCK STRUCTURES
- SPT DPT DMT VST PMT TEST BORING
- AUGER BORING
- CORE BORING
- MW MONITORING WELL
- PIEZOMETER INSTALLATION
- SLOPE INDICATOR INSTALLATION
- CONE PENETROMETER TEST
- SOUNDING ROD
- TEST BORING WITH CORE
- SPT N-VALUE

RECOMMENDATION SYMBOLS

- UNDERCUT
- SHALLOW UNDERCUT
- UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE
- UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK
- UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL

ABBREVIATIONS

AR - AUGER REFUSAL
BT - BORING TERMINATED
CL. - CLAY
CPT - CONE PENETRATION TEST
CSE. - COARSE
DMT - DILATOMETER TEST
DPT - DYNAMIC PENETRATION TEST
e - VOID RATIO
F - FINE
FOSS. - FOSSILIFEROUS
FRAC. - FRACTURED, FRACTURES
FRAGS. - FRAGMENTS
HI. - HIGHLY
MED. - MEDIUM
MICA. - MICACEOUS
MOD. - MODERATELY
NP - NON PLASTIC
ORG. - ORGANIC
PMT - PRESSUREMETER TEST
SAP. - SAPROLITIC
SD. - SAND, SANDY
SL. - SILT, SILTY
SLI. - SLIGHTLY
TCR - TRICONE REFUSAL
w - MOISTURE CONTENT
V - VERY
VST - VANE SHEAR TEST
WEA. - WEATHERED
γ - UNIT WEIGHT
γ_d - DRY UNIT WEIGHT

SAMPLE ABBREVIATIONS
S - BULK
SS - SPLIT SPOON
ST - SHELBY TUBE
RS - ROCK
RT - RECOMPACTED TRIAXIAL
CBR - CALIFORNIA BEARING RATIO

EQUIPMENT USED ON SUBJECT PROJECT

DRILL UNITS:
- [ ] CME-45C
- [ ] CME-55
- [X] CME-550X
- [ ] VANE SHEAR TEST
- [ ] PORTABLE HOIST
- [X] DIEDRICH D-50
- [ ] _____

ADVANCING TOOLS:
- [ ] CLAY BITS
- [ ] 6" CONTINUOUS FLIGHT AUGER
- [X] 8" HOLLOW AUGERS
- [ ] HARD FACED FINGER BITS
- [ ] TUNG.-CARBIDE INSERTS
- [X] CASING [X] W/ ADVANCER
- [ ] TRICONE ____" STEEL TEETH
- [ ] TRICONE ____" TUNG.-CARB.
- [X] CORE BIT
- [ ] _____

HAMMER TYPE:
- [X] AUTOMATIC
- [ ] MANUAL

CORE SIZE:
- [ ] -B _____
- [ ] -H _____
- [X] -N Q

HAND TOOLS:
- [ ] POST HOLE DIGGER
- [ ] HAND AUGER
- [ ] SOUNDING ROD
- [ ] VANE SHEAR TEST
- [ ] _____

ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.
ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:

WEATHERED ROCK (WR)	NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.
CRYSTALLINE ROCK (CR)	FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.
NON-CRYSTALLINE ROCK (NCR)	FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.
COASTAL PLAIN SEDIMENTARY ROCK (CP)	COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.

WEATHERING

FRESH	ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.
VERY SLIGHT (V SLI.)	ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.
SLIGHT (SLI.)	ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.
MODERATE (MOD.)	SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.
MODERATELY SEVERE (MOD. SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. *IF TESTED, WOULD YIELD SPT REFUSAL*
SEVERE (SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF*
VERY SEVERE (V SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF*
COMPLETE	ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.

ROCK HARDNESS

VERY HARD	CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.
HARD	CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.
MODERATELY HARD	CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.
MEDIUM HARD	CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.
SOFT	CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.
VERY SOFT	CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.

FRACTURE SPACING

TERM	SPACING
VERY WIDE	MORE THAN 10 FEET
WIDE	3 TO 10 FEET
MODERATELY CLOSE	1 TO 3 FEET
CLOSE	0.16 TO 1 FOOT
VERY CLOSE	LESS THAN 0.16 FEET

BEDDING

TERM	THICKNESS
VERY THICKLY BEDDED	4 FEET
THICKLY BEDDED	1.5 - 4 FEET
THINLY BEDDED	0.16 - 1.5 FEET
VERY THINLY BEDDED	0.03 - 0.16 FEET
THICKLY LAMINATED	0.008 - 0.03 FEET
THINLY LAMINATED	< 0.008 FEET

INDURATION

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.

FRIABLE	RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.
MODERATELY INDURATED	GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.
INDURATED	GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.
EXTREMELY INDURATED	SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS

ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.

AQUIFER - A WATER BEARING FORMATION OR STRATA.

ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.

ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.

ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.

CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.

COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.

CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.

DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.

DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.

FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.

FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.

FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.

FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.

FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.

JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.

LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.

LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.

MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.

PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.

RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.

ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.

SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.

SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.

STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.

STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.

STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.

TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

BENCH MARK:

ELEVATION: FEET

NOTES:

ROADWAY DESIGN FILES PROVIDED BY TGS DATED AUGUST 14, 2025.

BRIDGE BORING COLLAR ELEVATIONS OBTAINED USING CARLSON BRx7 GPS.

FIAD = FILLED IMMEDIATELY AFTER DRILLING.

NM = NOT MEASURED

BORINGS EB1-B AND EB2-A WERE INCLUDED IN A PREVIOUS DESIGN BUILD STRUCTURE SUBSURFACE INVESTIGATION REPORT AS BORINGS B-1 AND B-2, DATED DECEMBER 2024, PREPARED BY CG2. THE BORING DATA HAS BEEN RE-LABELLED AND INCORPORATED INTO THIS INVESTIGATION REPORT.

DATE: 8-15-14

PROJECT REFERENCE NO.	SHEET NO.
DF18312.2023011.PR	2A

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS

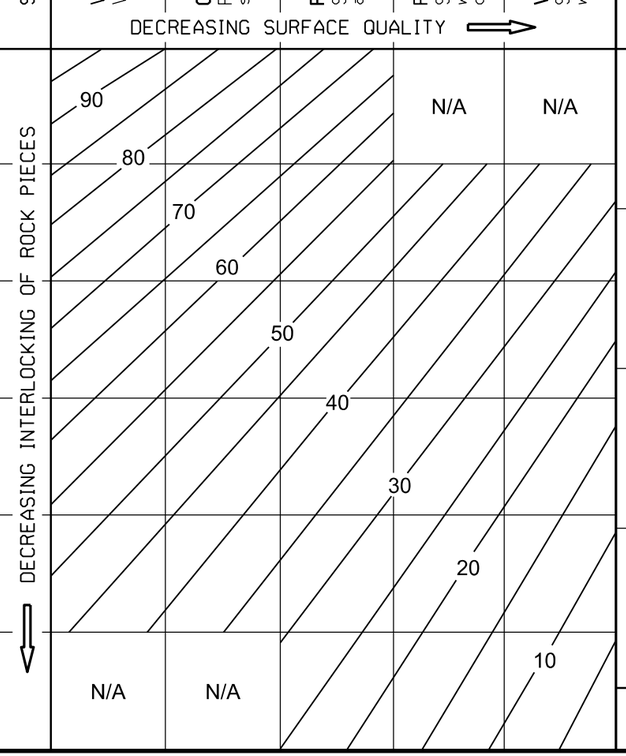
AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000)

GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)

From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.

STRUCTURE

- INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities
- BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets
- VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets
- BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity
- DISINTEGRATED - poorly interlocked, heavily broken rock mass with mixture of angular and rounded rock pieces
- LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes

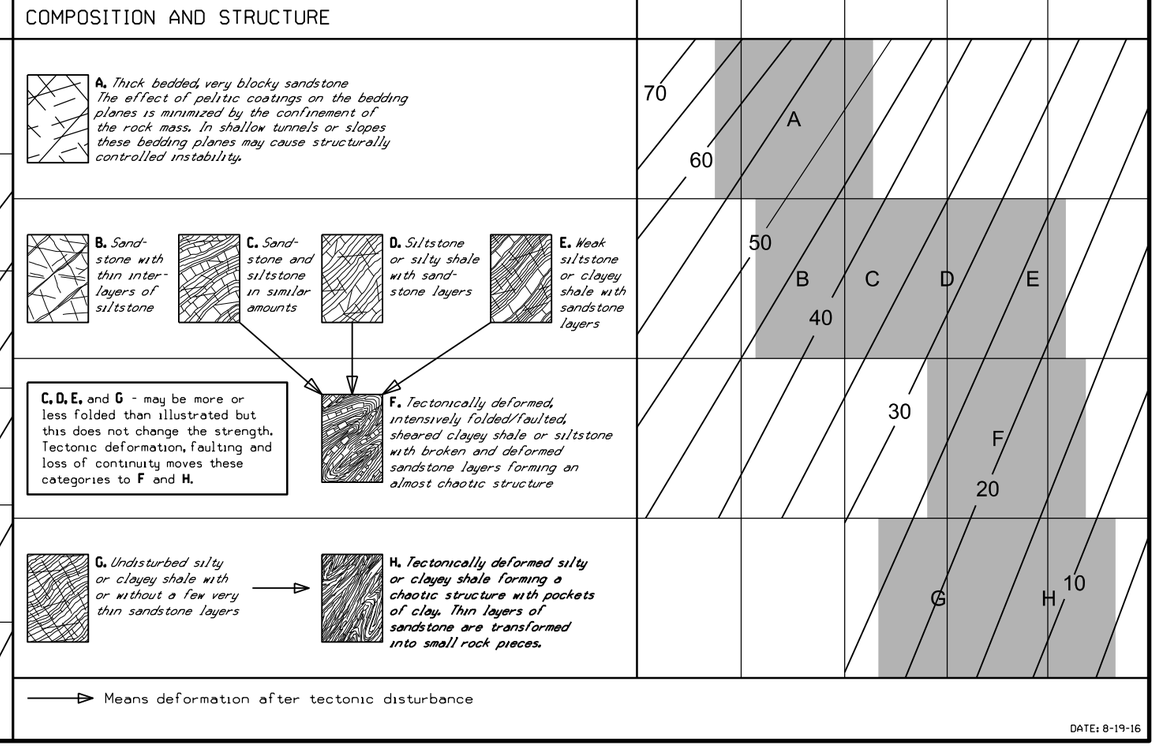


AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)

GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos. P and Hoek E., 2000)

From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.

COMPOSITION AND STRUCTURE



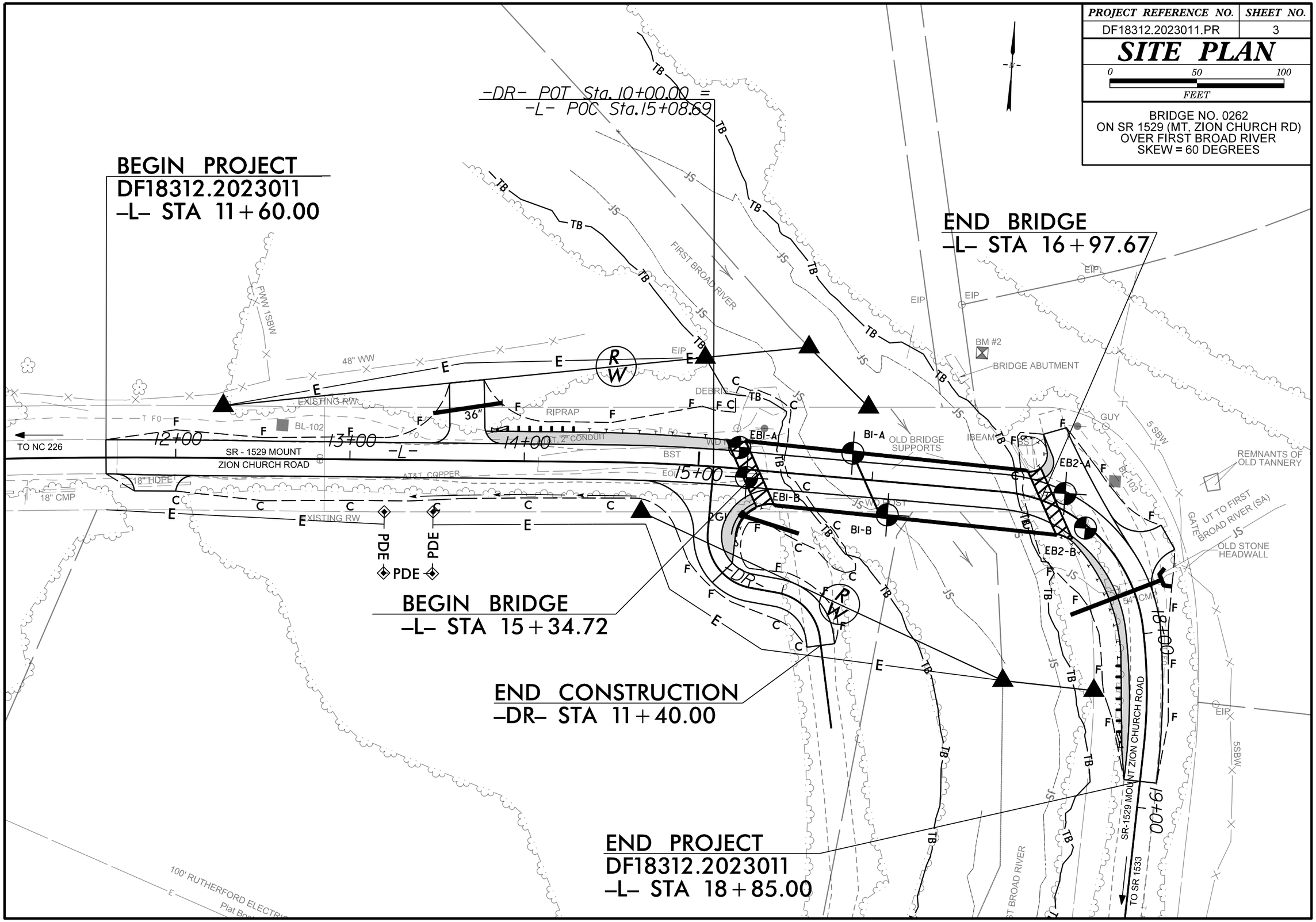
DATE: 8-19-16

PROJECT REFERENCE NO.	SHEET NO.
DF18312.2023011.PR	3

SITE PLAN

0 50 100
FEET

BRIDGE NO. 0262
ON SR 1529 (MT. ZION CHURCH RD)
OVER FIRST BROAD RIVER
SKEW = 60 DEGREES



GEOTECHNICAL BORING REPORT
BORE LOG

WBS DF18312.2023011.PR	**TIP** N/A	**COUNTY** CLEVELAND	**GEOLOGIST** R. Welch	
SITE DESCRIPTION Bridge No. 262 on SR 1529 (Mt. Zion Church Road) over First Broad River				**GROUND WTR (ft)**
BORING NO. EB1-A	**STATION** 15+23	**OFFSET** 12 ft LT	**ALIGNMENT** -L-	**0 HR.** N/A
COLLAR ELEV. 885.4 ft	**TOTAL DEPTH** 57.7 ft	**NORTHING** 638,740	**EASTING** 1,204,028	**24 HR.** Dry
DRILL RIG/HAMMER EFF./DATE CG20446 Diedrich D-50 97% 04/30/2024		**DRILL METHOD** NW Casing w/ SPT	**HAMMER TYPE** Automatic	
DRILLER C. Odom	**START DATE** 08/12/25	**COMP. DATE** 08/12/25	**SURFACE WATER DEPTH** N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION ELEV. (ft)	DEPTH (ft)
890											
885	885.4	0.0								885.4 GROUND SURFACE	0.0
			10	5	8	13		M		**ROADWAY EMBANKMENT** Stiff, Red-Brown-Gray, Fine Sandy SILT (A-4), with trace gravel	
	881.6	3.8								882.9	2.5
880			3	2	3	5		M		**ALLUVIAL** Very Loose to Medium Dense, Orange-Tan-Red-Brown-White-Gray, Silty Fine to Coarse SAND (A-2-4), with trace gravel and mica	
	876.6	8.8									
875			2	2	2	4		M			
	871.6	13.8									
870			2	2	1	3		W			
	866.6	18.8									
865			2	4	13	17		W			
	861.6	23.8								862.9	22.5
860			18	9	8	17		M		**RESIDUAL** Very Stiff, White-Orange-Gray, Fine to Coarse Sandy SILT (A-4), with trace mica	
	856.6	28.8								857.9	27.5
855			5	10	13	23		M		Medium Dense, White-Orange-Brown, Silty Fine to Coarse SAND (A-2-4), with trace mica	
	851.6	33.8								852.9	32.5
850			10	9	20	29		M		Very Stiff, White-Orange-Gray, Fine to Coarse Sandy SILT (A-4), with trace mica	
	846.6	38.8								846.6	38.8
845			22	78/0.4		100/0.9				**WEATHERED ROCK** White-Orange-Gray, (Mica Schist)	
	841.6	43.8								841.6	43.8
840			60/0.1			60/0.1				**CRYSTALLINE ROCK** White-Orange-Gray, (Mica Schist)	
	836.6	48.8								837.9	47.5
835			7	7	8	15		M		**RESIDUAL** Stiff, Brown-White, Fine Sandy SILT (A-4), with trace mica	
	831.6	53.8								831.6	53.8
830			75	25/0.1		100/0.6				**WEATHERED ROCK** Brown-Tan-White, (Mica Schist)	
	827.7	57.7	60/0.0			60/0.0				827.7	57.7
										Boring Terminated with Standard Penetration Test Refusal at Elevation 827.7 ft On Crystalline Rock (Mica Schist)	

WBS DF18312.2023011.PR	**TIP** N/A	**COUNTY** CLEVELAND	**GEOLOGIST** P. Tomasic	
SITE DESCRIPTION Bridge No. 262 on SR 1529 (Mount Zion Church Road) over First Broad River				**GROUND WTR (ft)**
BORING NO. EB1-B	**STATION** 15+28	**OFFSET** 5 ft RT	**ALIGNMENT** -L-	**0 HR.** NM
COLLAR ELEV. 886.1 ft	**TOTAL DEPTH** 70.6 ft	**NORTHING** 638,723	**EASTING** 1,204,033	**24 HR.** 13.9
DRILL RIG/HAMMER EFF./DATE CG23639 CME-550X 90% 03/10/2023		**DRILL METHOD** NW Casing w/ SPT	**HAMMER TYPE** Automatic	
DRILLER J. Kiker	**START DATE** 12/09/24	**COMP. DATE** 12/09/24	**SURFACE WATER DEPTH** N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
890										
										886.1 GROUND SURFACE 0.0
885	884.5	1.6								**ROADWAY EMBANKMENT** Medium Dense, Brown-Red, Silty Fine to Coarse SAND (A-2-4), with trace gravel
			9	11	9	20		M		883.0 3.1
	882.2	3.9								**ALLUVIAL** Loose, Red-Orange-Brown-Tan-Gray, Silty Fine to Coarse SAND (A-2-4), with trace mica, organics, and gravel
880			3	3	3	6		M		
	879.1	7.0								
			2	2	2	4		M		
	877.0	9.1								
875			2	2	2	4		M		
	872.1	14.0								
870			2	2	4	6		W		
	867.1	19.0								868.1 18.0
865			6	9	17	26		W		Medium Dense, Gray-White-Orange, Fine to Coarse SAND (A-3), with trace mica, quartz fragments, and gravel
	862.1	24.0								864.1 22.0
860			24	16	12	28		W		**RESIDUAL** Medium Dense to Very Dense, Gray-White-Orange, Silty Fine to Coarse SAND (A-2-4), with trace mica and quartz fragments
	857.1	29.0								
855			5	9	9	18		M		
	852.1	34.0								
850			13	12	9	21		M		
	847.1	39.0								
845			21	21	47	68		M		
	842.1	44.0								842.1 44.0
840			100/0.4			100/0.4				**WEATHERED ROCK** White-Orange-Gray, (Mica Schist)
										839.1 47.0
	837.1	49.0								**RESIDUAL** Medium Dense, Brown-White-Orange, Silty Fine to Coarse SAND (A-2-4), with trace mica and quartz fragments
835			1	8	8	16		M		
										833.1 53.0
	832.1	54.0								Very Stiff, Brown-Gray-White, Fine Sandy SILT (A-4), with trace to little mica
830			6	10	14	24		M		
	827.1	59.0								827.1 59.0
825			60	40/0.3		100/0.8				**WEATHERED ROCK** Brown-Gray-White, (Mica Schist)
	822.1	64.0								
820			100/0.4			100/0.4				
	817.1	69.0								
			100/0.2			100/0.2				
	815.5	70.6	60/0.0			60/0.0				815.5 70.6
										Boring Terminated with Standard Penetration Test Refusal at Elevation 815.5 ft In Crystalline Rock (Mica Schist)

Note:
Boring is Formerly identified as B-1 in the previous DB Structure Investigation Report, dated December 2024, prepared by CG2.

GEOTECHNICAL BORING REPORT
BORE LOG

WBS DF18312.2023011.PR	**TIP** N/A	**COUNTY** CLEVELAND	**GEOLOGIST** M. Malisher
SITE DESCRIPTION Bridge No. 262 on SR 1529 (Mt. Zion Church Road) over First Broad River			**GROUND WTR (ft)**
BORING NO. B1-A	**STATION** 15+88	**OFFSET** 15 ft LT	**ALIGNMENT** -L- / **0 HR.** N/A
COLLAR ELEV. 871.0 ft	**TOTAL DEPTH** 60.1 ft	**NORTHING** 638,741	**EASTING** 1,204,093 / **24 HR.** Caved
DRILL RIG/HAMMER EFF./DATE CG20446 Diedrich D-50 97% 04/30/2024		**DRILL METHOD** NW Casing W/SPT & Core	**HAMMER TYPE** Automatic
DRILLER C. Odom	**START DATE** 09/11/25	**COMP. DATE** 09/11/25	**SURFACE WATER DEPTH** N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION (ELEV. (ft) / DEPTH (ft))
875										
	871.0	0.0								871.0 GROUND SURFACE 0.0
870			WOH	WOH	1	1		Sat.		**ALLUVIAL** Very Loose to Loose, Brown-Tan, SAND (A-3)
	867.5	3.5	1	2	2	4		Sat.		
865										864.5 6.5
	862.5	8.5	2	2	2	4		W		**RESIDUAL** Soft to Stiff, Orange-Brown, Fine Sandy SILT (A-4), with trace mica
860										
	857.5	13.5	2	4	4	8		W		
855										
	852.5	18.5	3	4	8	12		M		
850										
	847.5	23.5	3	97/0.3		100/0.8				847.5 23.5 **WEATHERED ROCK** Orange-Tan-White, (Mica Schist)
845										
	842.5	28.5	100/0.3			100/0.3				
840										
	837.5	33.5	100/0.4			100/0.4				
835										
	832.5	38.5	100/0.4			100/0.4				
830	830.1	40.9	60/0.0			60/0.0				830.1 40.9 **CRYSTALLINE ROCK** Gray-White, (Mica Schist) REC = 98% RQD = 88% GSI = 70-75
825							RS-01			
820										
815							RS-02			
										810.9 60.1 Boring Terminated at Elevation 810.9 ft In Crystalline Rock (Mica Schist) Note: Boring was drilled in the channel; however, the water was shallow the day of drilling and surface water was not encountered at the boring location.

NCDOT BORE DOUBLE 22_GEO_BRDG0262_GTM.GPJ NC_DOT.GDT 11/12/25

GEOTECHNICAL BORING REPORT
CORE LOG

WBS DF18312.2023011.PR	**TIP** N/A	**COUNTY** CLEVELAND	**GEOLOGIST** M. Malisher
SITE DESCRIPTION Bridge No. 262 on SR 1529 (Mt. Zion Church Road) over First Broad River			**GROUND WTR (ft)**
BORING NO. B1-A	**STATION** 15+88	**OFFSET** 15 ft LT	**ALIGNMENT** -L- / **0 HR.** N/A
COLLAR ELEV. 871.0 ft	**TOTAL DEPTH** 60.1 ft	**NORTHING** 638,741	**EASTING** 1,204,093 / **24 HR.** Caved
DRILL RIG/HAMMER EFF./DATE CG20446 Diedrich D-50 97% 04/30/2024		**DRILL METHOD** NW Casing W/SPT & Core	**HAMMER TYPE** Automatic
DRILLER C. Odom	**START DATE** 09/11/25	**COMP. DATE** 09/11/25	**SURFACE WATER DEPTH** N/A
CORE SIZE NQ	**TOTAL RUN** 19.2 ft		

ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN REC. (ft) %	RUN RQD (ft) %	SAMP. NO.	STRATA REC. (ft) %	STRATA RQD (ft) %	LOG	DESCRIPTION AND REMARKS (ELEV. (ft) / DEPTH (ft))
830											Begin Coring @ 40.9 ft
830.1	830.1	40.9	4.2	*N=60/0.0* 6:26/1.0 4:27/1.0 6:45/1.0 9:49/1.0	(4.2) 100%	(3.8) 90%		(18.9) 98%	(16.9) 88%		830.1 40.9 **CRYSTALLINE ROCK** Slightly Weathered to Fresh, Medium Hard to Hard, Gray-White, (Mica Schist), with Close to Wide Fracture Spacing
	825.9	45.1									
825			5.0	2:05/0.2 9:46/1.0 6:58/1.0 3:46/1.0 2:59/1.0 2:27/1.0	(4.9) 98%	(3.9) 78%	RS-01				GSI = 70-75 RS-01: 45.1'-45.9' Unit Weight: 172.2 pcf Unconfined Compressive Strength: 14,930 psi (2,150 ksf)
	820.9	50.1									
820			5.0	2:20/1.0 3:52/1.0 5:05/1.0 4:42/1.0 3:29/1.0	(4.8) 96%	(4.6) 92%					RS-02: 56.4'-57.0' Unit Weight: 170.7 pcf Unconfined Compressive Strength: 12,910 psi (1,860 ksf)
	815.9	55.1									
815			5.0	3:31/1.0 3:10/1.0 6:12/1.0 4:00/1.0 2:13/1.0	(5.0) 100%	(4.6) 92%	RS-02				
	810.9	60.1									810.9 60.1 Boring Terminated at Elevation 810.9 ft In Crystalline Rock (Mica Schist) Note: Boring was drilled in the channel; however, the water was shallow the day of drilling and surface water was not encountered at the boring location.

NCDOT CORE DOUBLE 22_GEO_BRDG0262_GTM.GPJ NC_DOT.GDT 11/12/25

CAROLINAS GEOTECHNICAL GROUP

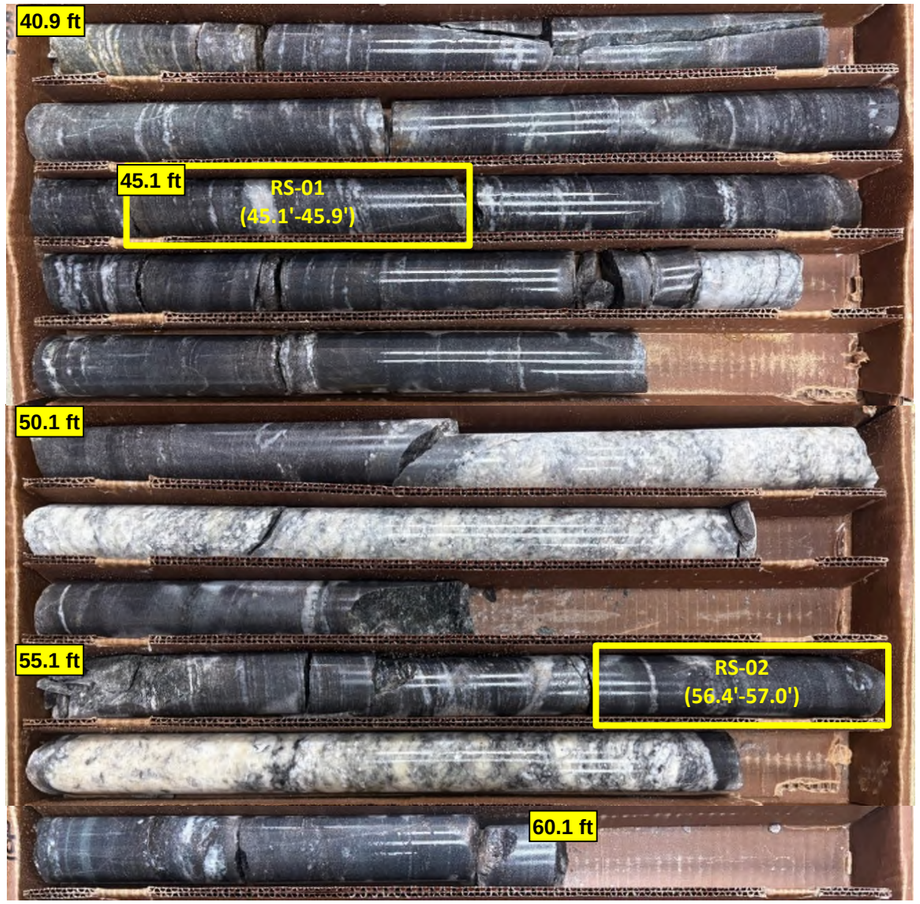
DF18312.2023011.PR - Bridge No. 0262 on SR 1529 (Mt. Zion Church Road) over First Broad River

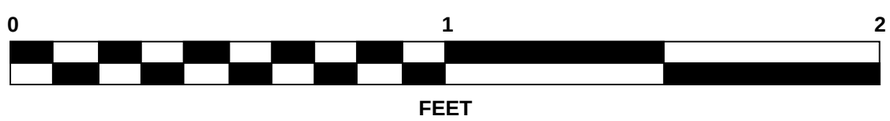
Cleveland County, North Carolina

Rock Core Photographs

B1-A

40.9 to 60.1 Feet





GEOTECHNICAL BORING REPORT
BORE LOG

WBS DF18312.2023011.PR	**TIP** N/A	**COUNTY** CLEVELAND	**GEOLOGIST** M. Malisher
SITE DESCRIPTION Bridge No. 262 on SR 1529 (Mt. Zion Church Road) over First Broad River			**GROUND WTR (ft)**
BORING NO. B1-B	**STATION** 16+10	**OFFSET** 19 ft RT	**ALIGNMENT** -L- / **0 HR.** N/A
COLLAR ELEV. 871.9 ft	**TOTAL DEPTH** 60.1 ft	**NORTHING** 638,707	**EASTING** 1,204,115 / **24 HR.** Caved
DRILL RIG/HAMMER EFF./DATE CG20446 Diedrich D-50 97% 04/30/2024		**DRILL METHOD** NW Casing W/SPT & Core	**HAMMER TYPE** Automatic
DRILLER C. Odom	**START DATE** 09/10/25	**COMP. DATE** 09/10/25	**SURFACE WATER DEPTH** N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION (ELEV. (ft) / DEPTH (ft))
875										
										871.9 / 0.0 GROUND SURFACE
870	870.9	1.0	2	2	1	3		M		**ALLUVIAL** Very Loose, Brown-Tan, Silty Fine SAND (A-2-4)
	868.1	3.8	2	3	3	6		Sat.		868.9 / 3.0 Loose, Brown-Tan, Fine to Coarse Sandy GRAVEL (A-1-a)
865										864.9 / 7.0 Dense, Gray-White, Fine to Coarse Gravelly SAND (A-1-b)
	863.1	8.8	5	9	32	41		Sat.		
860										859.9 / 12.0 **RESIDUAL** Stiff, Orange-Gray-Tan, Fine Sandy SILT (A-4), with trace mica
	858.1	13.8	2	2	7	9		Sat.		
855										
	853.1	18.8	25	51	49/0.3	100/0.8				852.6 / 19.3 **WEATHERED ROCK** Orange-Tan-Gray-White-Brown, (Mica Schist)
850										
	848.1	23.8	25	65	35/0.2	100/0.7				
845										
	843.1	28.8	100/0.3			100/0.3				
840										
	838.1	33.8	100/0.4			100/0.4				
835										
	833.8	38.1	60/0.0			60/0.0				833.8 / 38.1 **CRYSTALLINE ROCK** Gray, White, Orange, (Mica Schist) REC = 86% RQD = 68% GSI = 50-55
830										
825							RS-03			
820										
815							RS-04			811.8 / 60.1 Boring Terminated at Elevation 811.8 ft In Crystalline Rock (Mica Schist) Note: Boring was drilled in the channel; however, the water was shallow the day of drilling and surface water was not encountered at the boring location.

NCDOT BORE DOUBLE 22_GEO_BRDG0262_GTM.GPJ NC_DOT.GDT 10/29/25

GEOTECHNICAL BORING REPORT
CORE LOG

WBS DF18312.2023011.PR	**TIP** N/A	**COUNTY** CLEVELAND	**GEOLOGIST** M. Malisher
SITE DESCRIPTION Bridge No. 262 on SR 1529 (Mt. Zion Church Road) over First Broad River			**GROUND WTR (ft)**
BORING NO. B1-B	**STATION** 16+10	**OFFSET** 19 ft RT	**ALIGNMENT** -L- / **0 HR.** N/A
COLLAR ELEV. 871.9 ft	**TOTAL DEPTH** 60.1 ft	**NORTHING** 638,707	**EASTING** 1,204,115 / **24 HR.** Caved
DRILL RIG/HAMMER EFF./DATE CG20446 Diedrich D-50 97% 04/30/2024		**DRILL METHOD** NW Casing W/SPT & Core	**HAMMER TYPE** Automatic
DRILLER C. Odom	**START DATE** 09/10/25	**COMP. DATE** 09/10/25	**SURFACE WATER DEPTH** N/A
CORE SIZE NQ	**TOTAL RUN** 22.0 ft		

ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN REC. (ft) %	RUN RQD (ft) %	SAMP. NO.	STRATA REC. (ft) %	STRATA RQD (ft) %	LOG	DESCRIPTION AND REMARKS (ELEV. (ft) / DEPTH (ft))
833.8											Begin Coring @ 38.1 ft
	833.8	38.1	2.0	*N=60/0.0* 4:00/1.0 0:35/1.0	(0.8) 40%	(0.5) 25%		(19.0) 86%	(15.0) 68%		833.8 / 38.1 **CRYSTALLINE ROCK** Moderately Severe Weathering to Fresh, Medium Hard to Hard, Gray-White-Orange, (Mica Schist), with Close to Wide Fracture Spacing GSI = 50-55 RS-03: 44.0'-44.7' Unit Weight: 170.6 pcf Unconfined Compressive Strength: 16,590 psi (2,390 ksf) RS-04: 53.9'-54.7' Unit Weight: 173.9 pcf Unconfined Compressive Strength: 13,140 psi (1,890 ksf)
	831.8	40.1									
830			5.0	1:35/1.0 3:07/1.0 3:49/1.0 3:22/1.0 2:05/1.0	(3.2) 64%	(1.6) 32%					
	826.8	45.1					RS-03				
825			5.0	3:10/1.0 3:42/1.0 2:27/1.0 2:40/1.0 3:22/1.0	(5.0) 100%	(4.1) 82%					
	821.8	50.1									
820			5.0	2:49/1.0 3:04/1.0 3:00/1.0 3:16/1.0 3:45/1.0	(5.0) 100%	(4.6) 92%					
	816.8	55.1					RS-04				
815			5.0	2:58/1.0 3:28/1.0 3:35/1.0 3:24/1.0 3:58/1.0	(5.0) 100%	(4.2) 84%					
	811.8	60.1									811.8 / 60.1 Boring Terminated at Elevation 811.8 ft In Crystalline Rock (Mica Schist) Note: Boring was drilled in the channel; however, the water was shallow the day of drilling and surface water was not encountered at the boring location.

NCDOT CORE DOUBLE 22_GEO_BRDG0262_GTM.GPJ NC_DOT.GDT 10/29/25

CAROLINAS GEOTECHNICAL GROUP

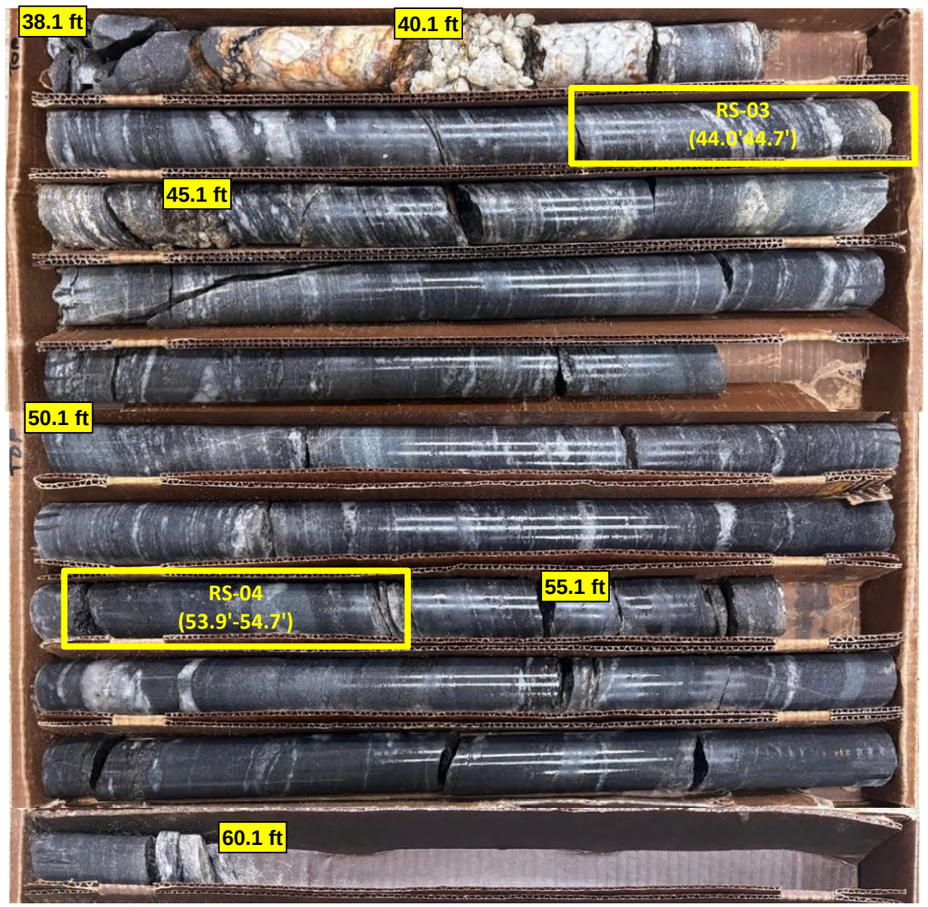
DF18312.2023011.PR - Bridge No. 0262 on SR 1529 (Mt. Zion Church Road) over First Broad River

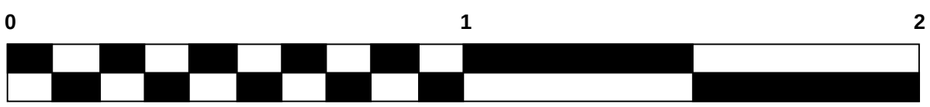
Cleveland County, North Carolina

Rock Core Photographs

B1-B

38.1 to 60.1 Feet





FEET

GEOTECHNICAL BORING REPORT
BORE LOG

WBS DF18312.2023011.PR	**TIP** N/A	**COUNTY** CLEVELAND	**GEOLOGIST** P. Tomasic
SITE DESCRIPTION Bridge No. 262 on SR 1529 (Mount Zion Church Road) over First Broad River			**GROUND WTR (ft)**
BORING NO. EB2-A	**STATION** 17+09	**OFFSET** 9 ft LT	**ALIGNMENT** -L- / **0 HR.** NM
COLLAR ELEV. 889.5 ft	**TOTAL DEPTH** 41.0 ft	**NORTHING** 638,726	**EASTING** 1,204,216 / **24 HR.** Caved
DRILL RIG/HAMMER EFF./DATE CG23639 CME-550X 90% 03/10/2023		**DRILL METHOD** NW Casing W/SPT & Core	**HAMMER TYPE** Automatic
DRILLER J. Kiker	**START DATE** 12/09/24	**COMP. DATE** 12/09/24	**SURFACE WATER DEPTH** N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION ELEV. (ft)	DEPTH (ft)
890										889.5 GROUND SURFACE	0.0
	888.1	1.4								**ROADWAY EMBANKMENT** Soft to Medium Stiff, Brown-Red, Fine to Coarse Sandy CLAY (A-6), with trace gravel and organics	
			3	2	3	5		M			
885	885.2	4.3									
			2	2	2	4		M		884.0	5.5
	882.9	6.6								**RESIDUAL** Very Stiff to Hard, Orange-Gray-White, Fine Sandy SILT (A-4), with trace mica and gravel-sized rock fragments	
			5	12	20	32		M			
880	880.7	8.8									
			20	14	12	26		M			
	876.0	13.5									
875			38	38	62/0.2					875.5	14.0
	873.5	16.0				100/0.7				**WEATHERED ROCK** Brown-White-Orange, (Pegmatite)	
			60/0.0			60/0.0				873.5	16.0
870										872.0 **CRYSTALLINE ROCK** Gray-White-Orange, (Pegmatite)	17.5
										REC = 100% RQD = 73% GSI = 55-60	
										Gray-White-Orange-Black, (Mica Schist)	
865										REC = 93% RQD = 49% GSI = 35-40	
860											
855										855.8	33.7
										White-Gray, (Pegmatite)	
										853.6	35.9
										REC = 100% RQD = 100% GSI = 85-90	
										851.4	38.1
850										Gray-Black, (Mica Schist)	
										849.4	40.1
										848.5	41.0
										REC = 100% RQD = 100% GSI = 85-90	
										White-Gray, (Pegmatite)	
										REC = 100% RQD = 100% GSI = 85-90	
										Gray-Black, (Mica Schist)	
										REC = 89% RQD = 89% GSI = 70-75	
										Boring Terminated at Elevation 848.5 ft In Crystalline Rock (Mica Schist)	

Note:

Could not measure 0 hr water level due to tooling breaking in ground during removal.

Boring is Formerly identified as B-2 in the previous DB Structure Investigation Report, dated December 2024, prepared by CG2.

GEOTECHNICAL BORING REPORT
CORE LOG

WBS DF18312.2023011.PR	**TIP** N/A	**COUNTY** CLEVELAND	**GEOLOGIST** P. Tomasic
SITE DESCRIPTION Bridge No. 262 on SR 1529 (Mount Zion Church Road) over First Broad River			**GROUND WTR (ft)**
BORING NO. EB2-A	**STATION** 17+09	**OFFSET** 9 ft LT	**ALIGNMENT** -L- / **0 HR.** NM
COLLAR ELEV. 889.5 ft	**TOTAL DEPTH** 41.0 ft	**NORTHING** 638,726	**EASTING** 1,204,216 / **24 HR.** Caved
DRILL RIG/HAMMER EFF./DATE CG23639 CME-550X 90% 03/10/2023		**DRILL METHOD** NW Casing W/SPT & Core	**HAMMER TYPE** Automatic
DRILLER J. Kiker	**START DATE** 12/09/24	**COMP. DATE** 12/09/24	**SURFACE WATER DEPTH** N/A
CORE SIZE NQ	**TOTAL RUN** 25.0 ft		

ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN REC. (ft) %	RUN RQD (ft) %	SAMP. NO.	STRATA REC. (ft) %	STRATA RQD (ft) %	LOG	DESCRIPTION AND REMARKS ELEV. (ft)	DEPTH (ft)
873.5											Begin Coring @ 16.0 ft	
	873.5	16.0	5.0	*N=60/0.0*	(4.7)	(2.8)		(1.5)	(1.1)		873.5 **CRYSTALLINE ROCK**	16.0
				2:15/1.0	94%	56%		100%	73%		872.0 Moderately to Slightly Weathered, Moderately Hard to Very Hard,	17.5
				2:36/1.0							Gray-White-Orange, (Pegmatite), with Very Close to Close Fracture	
870				1:59/1.0				(15.0)	(7.9)		Spacing	
				2:04/1.0				93%	49%			
	868.5	21.0		2:18/1.0							GSI = 55-60	
			5.0	2:03/1.0	(4.2)	(0.9)					Moderately Severely Weathered to Fresh, Medium Hard to Hard,	
				2:17/1.0	84%	18%					Gray-White-Black-Orange, (Mica Schist), with Very Close to Moderately	
				2:09/1.0							Close Fracture Spacing	
865				1:46/1.0								
	863.5	26.0		1:12/1.0							GSI = 35-40	
			5.0	1:54/1.0	(4.9)	(3.4)						
				2:14/1.0	98%	68%						
				1:35/1.0								
860				1:40/1.0								
	858.5	31.0		1:49/1.0								
			5.0	2:31/1.0	(5.0)	(4.2)						
				1:58/1.0	100%	84%						
				2:09/1.0								
855				5:05/1.0				(2.2)	(2.2)		855.8	33.7
	853.5	36.0		5:07/1.0				100%	100%		Very Slightly Weathered to Fresh, Hard to Very Hard, White-Gray,	
			5.0	3:09/1.0	(4.9)	(4.9)		(2.2)	(2.2)		(Pegmatite), with Moderately Close Fracture Spacing	
				2:05/1.0	98%	98%		100%	100%		853.6	35.9
				4:57/1.0							GSI = 85-90	
850				3:30/1.0				(2.0)	(2.0)		851.4	38.1
	848.5	41.0		2:29/1.0				100%	100%		Very Slightly Weathered to Fresh, Hard to Very Hard, Gray-Black, (Mica	
											Schist), with Moderately Close Fracture Spacing	
								(0.8)	(0.8)		849.4	40.1
								89%	89%		848.5	41.0
											GSI = 85-90	
											Very Slightly Weathered to Fresh, Hard to Very Hard, White-Gray,	
											(Pegmatite), with Moderately Close Fracture Spacing	
											GSI = 85-90	
											Very Slight Weathered to Fresh, Hard, Gray-Black, (Mica Schist), with	
											Moderately Close Fracture Spacing	
											GSI = 70-75	
											Boring Terminated at Elevation 848.5 ft In Crystalline Rock (Mica Schist)	

Note:

Could not measure 0 hr water level due to tooling breaking in ground during removal.

Boring is Formerly identified as B-2 in the previous DB Structure Investigation Report, dated December 2024, prepared by CG2.

CAROLINAS GEOTECHNICAL GROUP

DF18312.2023011.PR - Bridge No. 0262 on SR 1529 (Mt. Zion Church Road) over First Broad River

Cleveland County, North Carolina
Rock Core Photographs
B-2
16.0 to 41.0

GEOTECHNICAL BORING REPORT
BORE LOG

WBS DF18312.2023011.PR	**TIP** N/A	**COUNTY** CLEVELAND	**GEOLOGIST** K. Clark
SITE DESCRIPTION Bridge No. 262 on SR 1529 (Mt. Zion Church Road) over First Broad River			**GROUND WTR (ft)**
BORING NO. EB2-B	**STATION** 17+28	**OFFSET** 1 ft RT	**ALIGNMENT** -L-	**0 HR.** 15.2
COLLAR ELEV. 890.0 ft	**TOTAL DEPTH** 18.5 ft	**NORTHING** 638,707	**EASTING** 1,204,229	**24 HR.** FIAD
DRILL RIG/HAMMER EFF./DATE CG24113 CME-550X 78% 05/06/2024		**DRILL METHOD** H.S. Augers	**HAMMER TYPE** Automatic
DRILLER J. Estep	**START DATE** 08/29/25	**COMP. DATE** 08/29/25	**SURFACE WATER DEPTH** N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION ELEV. (ft)	DEPTH (ft)
890										890.0 GROUND SURFACE	0.0
	888.4	1.6								**ROADWAY EMBANKMENT** Medium Stiff, Red-Brown, Fine to Coarse Sandy SILT (A-4), with trace mica and organics	
	886.8	3.2	2	3	3	6		M		887.0	3.0
885	884.7	5.3	WOH	2	3	5		W		**RESIDUAL** Medium Stiff to Stiff, Red-Orange, Silty CLAY (A-7), with trace mica and gravel	
			4	4	5	9		W		882.5	7.5
	881.7	8.3								Stiff, Orange-Brown-Tan, Fine to Coarse Sandy SILT (A-4), trace to little mica and gravel-sized rock fragments	
880			3	4	5	9		M			
	876.7	13.3									
875			3	4	8	12		M			
	871.9	18.1								871.9	18.1
	871.5	18.5	60/0.1			60/0.1				871.5 **CRYSTALLINE ROCK** (Mica Schist)	18.5
			60/0.0			60/0.0				Boring Terminated with Standard Penetration Test Refusal at Elevation 871.5 ft In Crystalline Rock (Mica Schist)	
										Surficial Organic Soil: 0.2'	

ROCK TEST RESULTS

SAMPLE NO.	BORING	NORTHING	EASTING	STATION	OFFSET	DEPTH INTERVAL	ROCK TYPE	UNIT WEIGHT (PCF)	UNCONFINED COMPRESSIVE STRENGTH
RS-01	B1-A	638741	1204093	15+88 -L-	15' LT	45.1'-45.9'	MICA SCHIST	172.2	14,930 psi/2,150 ksf
RS-02	B1-A	638741	1204093	15+88 -L-	15' LT	56.4'-57.0'	MICA SCHIST	170.7	12,910 psi/1,860 ksf
RS-03	B1-B	638707	1204115	16+10 -L-	19' RT	44.0'-44.7'	MICA SCHIST	170.6	16,590 psi/2,390 ksf
RS-04	B1-B	638707	1204115	16+10 -L-	19' RT	53.9'-54.7'	MICA SCHIST	173.9	13,140 psi/1,890 ksf

AUTHORIZED SIGNATURE
NCDOT CERT NO. 130-04-0212

Prepared in the Office of:
F&ME CONSULTANTS, INC.
COLUMBIA, SOUTH CAROLINA
NCDOT LAB CERT. NO. 130-0212

PROJECT REFERENCE NO.	SHEET NO.
DF18312.2023011.PR	13

SITE PHOTOS

PHOTO #1: VIEW FROM END BENT NO. 1 FACING EAST TOWARD END BENT NO. 2.

PHOTO #2: VIEW FROM END BENT NO. 2 FACING WEST TOWARD END BENT NO. 1.